#### FULL SCALE EARTHQUAKE RESPONSE

#### OF A REHABILITATED BUILDING

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#### SUMMARY

The ambient, harmonic vibrator and earthquake response of a three story concrete shear wall building has recorded and analyzed. The harmonic response was measured prior to and after a major strengthing rehabilitation. This paper summarizes the response data analysis to date.

#### INTRODUCTION

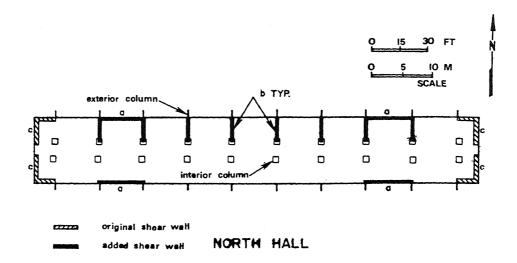
This building is one of several under study at the UCLA Full Scale Earthquake and Wind Laboratory. The accompaning figures and tables describe a three story building which has experienced various dynamic environments. The building has been studied both before and after rehabilitation which added increased lateral force resistance. One earthquake induced response of the building. The earthquake was the Richter magnitude 5.8, August 13, 1978, Santa Barbara earthquake.

#### **OBSERVATIONS**

The accompaning tables and figures speak for themselves. References are cited for those interested in performing further study of the building. The fundamental observations are the following: (1) The earthquake base shear was approximately 2.5 times the design base shear and was approximately 50% of the building weight; (2) The ground floor level experience measurable and significant twisting earthquake motion; (3) Intrastory drifts were approximately 0.3 percent of the story height and no noticable structural damage was experienced; (4) Floor fundamental mode motion was a combination of translation, torsion and inplane slab deformation; (5) Ground level input motion was slightly less than 1940 El Centro intensity translational and twisting input motion,

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Floor plan of North Hall, UCSB.

Figure 1\*

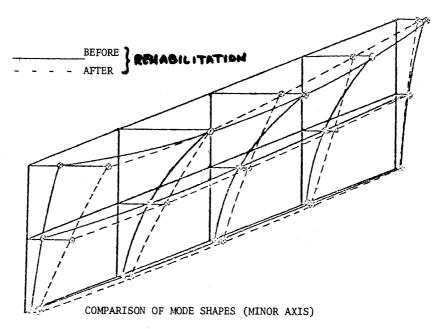
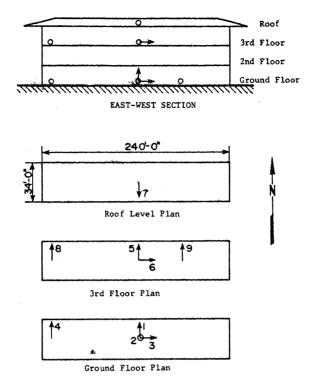


Figure 2



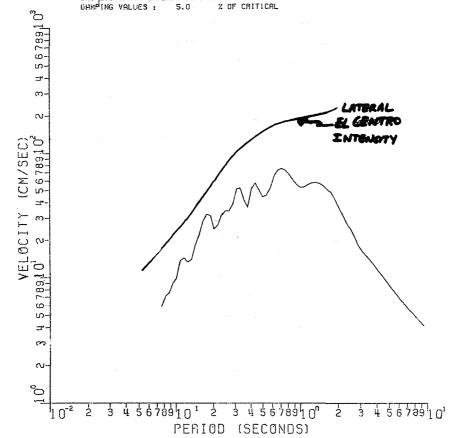
Location of accelerometers within North Hall, UCSB.

LOCATION OF ACCELEROMETERS WITHIN NORTH HALL, UCSB

Figure 3

## RESPONSE SPECTRUM

SANTS BORBORRS EARTHOUGKE OF 13 AUGUST 1978 2254 DMG 213 SONIS BARBARA UCSB NORTH HALL S/N 115 TR. 1 DAMFING VALUES: 5.0 % OF CRITICAL



RESPONSE SPECTRUM

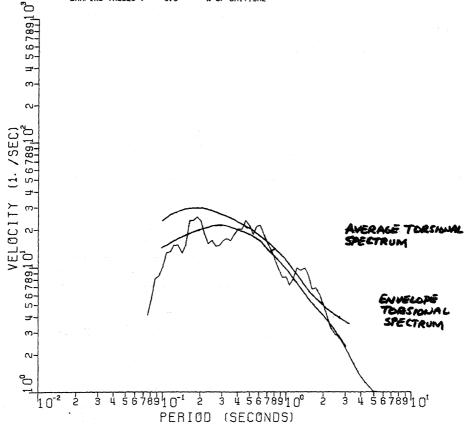
Figure 4

# RESPONSE SPECTRUM

SANTA BARBARA EARTHQUAKE OF 13 AUGUST 1978 2254

DMG 213 SANTA BARBARA UCSB NORTH HALL S/N 115 1000 x (TR.4 - TR.17 / XDL

DAMPING VALUES: 5.0 % OF CRITICAL



RESPONSE SPECTRUM

Figure 5

Table 1: Harmonic Vibration Tests:
Maximum Measured Acceleration

	North-South	East-West	Torsion
Pre-Rehabilitation Harmonic Vibration	3%g	3%g	3%g
Post-Rehabilitation Harmonic Vibration	1%g	0.6%g	1%g

Table 2: Harmonic Vibration Tests:
Percent Critical Damping Ratio

	North-South	East-West	Torsion
Pre-Rehabilitation Harmonic Vibration	6.8 - 7.5	9.4 - 11.8	10.3 - 11.4
Post-Rehabilitation Harmonic Vibration	6.1 - 6.3	9.3 - 11.9	

Table 3: Harmonic Vibration Tests:
Outside Building Response

	West	Side	Eas	t Side	Sc	outh Side	
	5 Ft.	25 Ft.	5 Ft.	25 Ft.	5 Ft.	25 Ft.	45 Ft.
Minor	15.5*	4.6	1.5	0+	10.1	9.3	+
Major	+	+	+	+	19.1	11.3	10.4
Torsion	44.7	14.5	2.5	0 <sup>+</sup>	10.4	10.3	+

Table 4: Building Natural Frequencies (Hz)

	North-South	East-West	Torsion
Pre-Rehabilitation Harmonic Vibration	3.5 - 3.7	3.9 - 4.1	4.8 - 4.9
Post-Rehabilitation Harmonic Vibration	4.9	6.1	5.7 - 6.1
Post-Earthquake Ambient Vibration	4.0	4.9	6.5

Note: \*: All numbers shown are percent of reference accelerometer at 3rd level.

<sup>+:</sup> Means do data was taken.

Table 5: Building Earthquake Motion

	T =	
Peak Accel. (Time)	Peak Velocity (Time)	Peak Displacement (Time)
	·	
351.11 (4.50)	-35.45 (4.36)	5.91 (4.12)
395.69 (4.50)	-35.53 (4.34)	6.19 (4.10)
99.13 (5.88)	3.67 (4.24)	-0.45 (13.98)
-534.49 (4.72)	-48.26 (4.40)	6.56 (4.06)
679.63 (4.88)	-46.44 (4.40)	7.36 (4.06)
-363.68 (4.16)	11.19 (4.12)	-0.83 (4.04)
		T.
-534.49 (4.72)	-48.26 (4.40)	6.56 (4.06)
351.11 (4.50)	-35.45 (4.36)	5.91 (4.12)
-376.16 (4.72)	-17.48 (4.78)	-1.12 (4.88)
	,	
679.63 (4.88)	-46.44 (4.40)	7.36 (4.06)
395.69 (4.50)	-35.53 (4.34)	6.19 (4.10)
-643.52 (4.04)	-24.26 (4.80)	-1.33 (4.88)
-975.90 (4.06)	-54.34 (3.98)	+8.21 (4.06)
395.69 (4.50)	-35.53 (4.34)	6.19 (4.10)
-945.13 (4.06)	39.95 (4.00)	2.13 (4.06)
-975.90 (4.06)	+54.34 (3.98)	+8.21 (4.06)
679.63 (4.88)	-46.44 (4.40)	7.36 (4.06)
-448.88 (4.08)	18.59 (4.00)	0.86 (4.74)
	395.69 (4.50) 99.13 (5.88)  -534.49 (4.72) 679.63 (4.88) -363.68 (4.16)  -534.49 (4.72) 351.11 (4.50) -376.16 (4.72)  679.63 (4.88) 395.69 (4.50) -643.52 (4.04)  -975.90 (4.06) 395.69 (4.50) -945.13 (4.06)  -975.90 (4.06) 679.63 (4.88)	351.11 (4.50) 395.69 (4.50) 99.13 (5.88)  -35.53 (4.34) 99.13 (5.88)  -48.26 (4.40) -534.49 (4.72) -534.49 (4.72) -534.49 (4.72) -534.49 (4.72) -534.49 (4.72) -534.49 (4.72) -48.26 (4.40) -363.68 (4.16)  11.19 (4.12)  -534.49 (4.72) -48.26 (4.40) -351.11 (4.50) -35.45 (4.36) -376.16 (4.72)  -17.48 (4.78)  -46.44 (4.40) -395.69 (4.50) -35.53 (4.34) -643.52 (4.04)  -975.90 (4.06) -945.13 (4.06)  -975.90 (4.06)

Note: Acceleration Units (cm/sec<sup>2</sup>)

Velocity Units (cm/sec)

Displacement Units (cm)

Time Units (seconds after start of record)

Table 6: Base Shear Magnitudes

Direction	Method	V/W
North-	UBC 1976	0.28
South	ATC-3	0.15
East- West	UBC 1976	0.28
	ATC-3	0.15
Santa Barbara Earthquake		0.60

### REFERENCES

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- Hart, G.C., Huang, S., Thomson, W.T., Torkamani, M.A.M. and Rea, D., "Forced Vibration Testing of a Rehabitated Multi-story Building", University of California, Los Angeles, Technical Report UCLA - ENG - 7822, June 1978.
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